US 6 Bridges Design Build Project

BR 0061-083 Sub Account Number 18838 (CN)

Master Plan Water Quality Report

Prepared for: Colorado Department of Transportation Federal Highway Administration

Prepared by:

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Water Quality Mater Plan Report Mitigation

Appropriate measures to minimize environmental harm from the Project have been identified. Mitigation measures for water resources in the FEIS and 2007 ROD were reviewed and carried forward if applicable, and mitigation measures were added for any resources with additional impacts. A summary of all mitigation measures for Phase 1 (including the US 6/Federal Boulevard interchange) as presented in the 2007 ROD, Phase 5 as presented in the FEIS, and the six new project elements are shown in Table 1 below. These mitigation measures are consistent with both the CDOT Mitigation Tracking Form in Appendix C and Book 2 Section 5 of the Project contract documents.

	EIS and ROD		US 6 Bridges Design	US 6 Bridges Design Build Project	
Resource	Impacts of Proposed Action	Mitigation	Build Project: What Has Changed	Impacts of Proposed Action	Mitigation
Water Quality and Water Resources	Short-term increase in sediment from construction. Increase in impervious drainage area. Consolidation of stormwater runoff with fewer outfalls to the South Platte River. Improved quality of stormwater discharge due to construction of water quality ponds and best management practice (BMP) stormwater facilities.	Use construction BMPs to reduce temporary impacts. On-site project area runoff will be controlled through water quality ponds or other BMPs to settle and improve water quality runoff releasing to the South Platte River. Reduction of the overall number of outfalls into the South Platte River and installation of energy dissipaters, such as riprap, at outfalls to reduce erosion potential. Use pump stations to remove runoff at underpasses on grade separations and use water quality ponds to settle sediment and improve water quality releasing into the South Platte River.	Two new water quality ponds.	Improvement to water quality and stormwater management due to new ponds.	Implement appropriate temporary BMPs for erosion and sediment control according to the CDOT Erosion Control and Stormwater Quality Guide (CDOT, 2002), and develop a stormwater management plan (SWMP), which includes water quality monitoring by the construction contractor to ensure effectiveness of temporary construction BMPs. Provide for permanent stabilization consistent with CDOT's MS4 permit through revegetation and permanent erosion controls measures. Identify hazardous spill containment structure locations and recommend BMPs based on their potential effectiveness in reducing hazardous waste discharge to the South Platte River. Comply with CDOT Standard Specification 207 and 208.

Table 1. Summary of Previously and Currently Identified Water Resource Impacts and Mitigation

	EIS and ROD		US 6 Bridges Design	US 6 Bridges Design Build Project	
Resource	Impacts of Proposed Action	Mitigation	Build Project: What Has Changed	Impacts of Proposed Action	Mitigation
					Use storm sewer system, pump stations, or other approved methods to remove runoff at underpasses on grade separations and use water quality ponds or other approved water quality BMPs to settle sediment and improve water quality prior to releasing the runoff into the South Platte River. Reduce the overall number of outfalls into the South Platte River in compliance with CDOT's MS4 permit. Install energy dissipaters, such as riprap, or other equitable allowable BMPs, at outfalls to reduce erosion potential in accordance with Section 208 of the 2011 Standard Specification for Road and Bridge Construction. The 2012 Reevaluation and preliminary design identified the need for water quality ponds. Construct ponds or other equitable allowable permanent BMPS, for erosion and sediment control according the CDOT Erosion Control and Stormwater Quality Guide (CDOT, 2002).

	EIS and ROD		US 6 Bridges Design	US 6 Bridges Design Build Project	
Resource	Impacts of Proposed Action Mitigation		Build Project: What Has Changed	Impacts of Proposed Action	Mitigation
	Temporary impacts during replacement of SB Santa Fe Drive and Alameda Avenue bridges over the South Platte River.	Design bridges to minimize the impact on floodplains of piers, abutments, and roadways, to the extent practicable.	Two new water quality ponds.	Water quality and stormwater management improvement due to new ponds	Design bridges to minimize the impact on floodplains from piers, abutments, and roadways, to the extent practicable. Restore construction areas to the pre-construction conditions in
Floodplains	from SB I-25 off ramp to Santa Fe Drive.	Install storm sewer improvements to reduce flooding on I-25 under Alameda Avenue.			Provide adequate floodplain width in areas of floodplain encroachment for overall "no rise" in floodplain.
		Provide adequate floodplain width in areas of floodplain encroachment for overall "no rise" in floodplain.			

Master Plan Water Quality Report

US 6 Bridges Design Build Project BR 0061-083 Sub Account No. 18838 (CN) Denver, Colorado

Prepared for Colorado Department of Transportation

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5 1.0 INTRODUCTION

6 This Water Quality Report presents the water quality concepts, design calculations, permanent 7 stormwater best management practices (BMPs) for planned bridge and highway improvements 8 of US 6 from Knox Court to the Burlington Northern Santa Fe (BNSF) Railroad bridge to be 9 constructed as a design build project. This report is prepared in accordance with the Colorado 10 Department of Transportation (CDOT) Drainage Design Manual (DDM), the City and County of 11 Denver (CCD) Storm Drainage Design & Technical Criteria Manual (SDDTCM), the Urban 12 Drainage Flood Control District (UDFCD) Urban Storm Drainage Criteria Manual (USDCM), and 13 standard engineering practices.

14 **1.1 Location of Improvements**

The overall project area, shown in Figure 1, is located in the south halves of Sections 4 and 5
and the north halves of Sections 8 and 9, Township 4 South, Range 68 West of the Sixth
Principal Meridian in the City and County of Denver, Colorado. The project will include highway
improvements along US 6 from Knox Court to the BNSF Bridge east of Interstate 25 (I-25);
along I-25 from approximately 700 feet north of the US 6 interchange to 1300 feet south of the
interchange; along Federal Boulevard from 5th Avenue to 7th Avenue; along 5th Avenue from

21 Federal Boulevard to Decatur Street; and reconstruction of Barnum Park East.

22 The receiving water for the project is the South Platte River.

23 **1.2 Description of Improvements**

- 24 The planned highway improvements consist of replacing the bridges at Federal Boulevard,
- 25 Bryant Street, the South Platte River, I-25, and the BNSF Railroad, and associated highway and
- 26 ramp improvements. Barnum Park East, located at the southeast corner of the Federal
- 27 Boulevard/US 6 Interchange, will also be reconstructed with the project.



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28 29

Figure 1: Vicinity Map

30 2.0 WATER QUALITY REQUIREMENTS

31 **2.1** Adherence to the CDOT MS4 Permit and NDRD Requirements

32 CDOT's MS4 permit and New Development and Redevelopment (NDRD) Stormwater

33 Management Program require that BMPs be provided for significant highway redevelopment 34 construction that will disturb more than 1 acre.

NDRD requires that development and redevelopment projects provide 100% water quality
 capture volume or BMPs to remove 80% of the total suspended solids.

37 2.2 Assumptions and Methodologies

"Offset area" is defined as non-project impervious area treated by permanent stormwater BMPs
 to offset project impervious area in a highly constrained area that cannot be treated. "Highly

40 constrained" is defined as an area where water quality treatment cannot be provided due to

41 public safety concerns, maintenance crew safety concerns, stormwater pumping requirements,

42 utility conflicts, elevation constraints, space constraints, or other factors which limit the ability to

43 install and maintain BMPs.



44 **3.0 WATER QUALITY DESIGN**

45 **3.1 Design Coordination**

- 46 3.1.1 Adjacent Segments
- 47 The EIS delineated a basin on I-25 to the south of this project, designated the "3rd Avenue
- 48 Basin," located on I-25 between the I-25 Bridge over the BNSF railroad and the I-25 & Alameda
- 49 project. It proposed that the water quality flows from the 3^{rd} Avenue basin be conveyed to the 6^{th}

50 Avenue Interchange Water Quality Pond. The 3rd Avenue basin will be constructed by others. 51 However, the 6th Avenue Interchange Water Quality Pond, constructed with this project, will

51 However, the 6th Avenue Interchange Water Quality Pond, constructed with this project 52 need to be sized to provide water quality capture volume for the 3rd Avenue basin.

- 53 3.1.2 Agency Coordination
- 54 Design approvals will be required from CDOT and CCD.

55 3.2 Hydrology and Design Flow Development

56 The Rational Method model was used to estimate the peak runoff rates for the water quality

- 57 storm. This method is defined as:
- 58 Q = CIA
- 59 where: Q = Peak discharge in cubic feet per second (cfs)
- 60 C = Runoff coefficient
- 61 I = Rainfall intensity in inches per hour
- 62 A = Drainage area in acres

63 A Water Quality storm event with a one-hour point rainfall value of 0.6 inches was used for

- 64 design, which is consistent with UDFCD and CCD criteria, instead of 0.5-inches in the CDOT
- 65 criteria.

66 The time of concentration is required to select the appropriate rainfall intensity. The time of 67 concentration was calculated using equations from Section 2.4 of the Runoff chapter of the USDCM. The resultant time found using these equations is compared to the time of 68 69 concentration for urbanized basins, which is also calculated using an equation from Section 2.4. 70 The lesser of the two times is used to determine the rainfall intensity, with a minimum allowable 71 time of concentration equal to five minutes. Runoff coefficients were determined using the 72 percent imperviousness, NRCS hydrologic soil group, and Table RO-3 from the USDCM. 73 Equation 5.1 from the SDDTCM was used to determine the rainfall intensities for each sub-74 basin. Manning's equation for full pipe flow was used to calculate pipe flow velocities and travel 75 times. Figure 7.2 from the DDM was used to calculate overland flow travel times for the 100-76 year storm event.



77 **3.3 Description of Water Quality Basins**

78 The project area is divided into fifteen water quality basins, labeled WQ-1 through WQ-15.

At Knox Court, the west extent of the project, runoff from non-project area flows east into the project area. WQ-1 includes both non-project and project area, consisting of all the tributary area upstream of the proposed water quality diversions near the Weir Gulch culvert under US 6 (see Section 3.4.2 for further discussion of the water quality diversions). This water quality basin is comprised of 2.29 acres of project impervious area and 31.83 acres of non-project impervious area.

- 85 Basin WQ-2 includes an area on US 6 between Knox Court and Federal Boulevard which
- discharges to Weir Gulch. This basin includes 2.68 acres of project impervious area and 0 acres
 of non-project impervious area.
- 88 Basin WQ-3 includes the project area drained by the US 6 Federal to Bryant storm drain
- 89 system. This basin includes US 6 from the Weir Gulch culvert under US 6 to Bryant Street,
- 90 Federal Boulevard from West 5th Avenue to West 7th Avenue, W 5th Avenue from Federal to
- 91 North Decatur Street, and Barnum Park East. WQ-3 includes 23.84 acres of project impervious
- 92 area and 4.67 acres of non-project impervious area.
- 93 Basin WQ-4 is 0.34 acres of the eastbound collector/distributor ramp to Bryant Street. This
- basin is lower in elevation than the US 6 Water Quality Pond; therefore runoff from this area is
- 95 not intercepted by the storm drain system and flows east.
- The embankment of the US 6 Water Quality Pond is a grassy slope on the downstream side of the pond. This basin, WQ-5, contains 0.41 acres of pervious area and 0 acres of impervious
- 98 area. No treatment is needed.
- At the north extent of the project area on Federal Boulevard is 0.17 acres of impervious project
- area and 0 acres of non-project impervious area. Runoff from this area, WQ-6, flows north of the
- 101 proposed inlets.
- 102 Basin WQ-7, which includes 0.48 acres of impervious project area and 0 acres of non-project
- 103 impervious area, drains south to and is treated by the existing Barnum Lake Constructed
- 104 Wetland Pond constructed with the Federal Boulevard Improvement Project: Alameda to 6th
- 105 Avenue.
- 106 Basin WQ-8 includes the westbound US 6 frontage road, on the north side of US 6 between the
- 107 US 6 Water Quality Pond and Alcott Street. This area is 0.73 acres of project impervious area
- and 0 acres of non-project impervious area. As it is lower in elevation than the storm drain
- system to US 6 Water Quality Pond, runoff flows east beyond the project limits.
- 110 US 6 between Bryant Street and the east abutment of the I-25 collector/distributor tunnel drains
- to the South Platte River. This basin, WQ-9, is comprised of 6.34 acres of project impervious
- 112 area and 0 acres of non-project impervious area.
- 113 Basin WQ-10 includes the eastbound US 6 to southbound I-25 ramp, from Bryant Street to the
- South Platte River. This basin contains 1.13 acres of impervious project area and 0 acres of
- 115 non-project impervious area.



- 116 I-25 between 8th Avenue and US 6, Ramps "A,' "B," and "C," and a portion of US 6 at the bridge
- over I-25 are included in Basin WQ-11. This basin contains 9.23 acres of project impervious
 area and 1.04 acres of non-project impervious area.
- Basin WQ-12 consists of Ramp "D," US 6 between the BNSF bridge and the I-25
- 120 collector/distributor tunnel, a portion of I-25 between US 6 and the BNSF bridge to the south,
- and the area under the flyover ramp between Ramp "C" and I-25. This water quality basin
- 122 consists of 5.66 acres of project impervious area and 1.01 acres of non-project impervious area.
- Basin WQ-13 consists of 1.98 acres of project impervious area and 0 acres of non-project
 impervious area on US 6 from the BNSF bridge to the east extent of the project.
- 125 The embankment of the north and east sides of Ramp "B" is a grassy slope. This basin, WQ-14, 126 contains 0.80 acres of pervious area and 0 acres of impervious area. No treatment is needed.
- 127 The embankment on the west and south sides of Ramp "C" is a grassy slope. This basin, WQ-
- 128 15, contains 0.37 acres of pervious area and 0 acres of impervious area. No treatment is 129 needed.
- 130 See Appendix B for the Water Quality Concept Plan.

131 **3.4 Permanent Stormwater Best Management Practices**

- 132 The US 6 Water Quality Pond and the 6th Avenue Interchange Water Quality Pond are proposed
- 133 extended detention basins (EDB) that will be used to provide water quality for the project area.
- 134 Locations for the water quality ponds are shown in Figure 2 below.
- 135 Due to site constraints, EDB and underground vaults are the only viable water quality treatment
- 136 options. Vaults were not considered due to safety concerns to maintenance crews and the
- 137 public. Infiltration basins, infiltration trenches, and filtering basins were not considered due to
- high groundwater levels. Wet ponds and constructed stormwater wetlands are not a viable
- 139 option due to the lack of a constant source of flow.
- 140 Extended detention basins have an expected pollutant removal rate of 55-75% TSS removal, as
- 141 stated in Table 4.7 of the New Development and Redevelopment Stormwater Management
- 142 Program Manual, CDOT, February 2004.
- 143 There are no existing water quality features within the project area.

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- Figure 2: Locations of BMPs
- 147 3.4.1 Water Quality Treatment

148 Water quality treatment for Basins WQ-1 and WQ-3 will be provided by the US 6 Water Quality

149 Pond. Basin WQ-12 will be treated by the 6th Avenue Interchange Water Quality Pond. See

150 Sections 3.4.2 and 3.4.3 for further discussion of these ponds.

151 Basins WQ-5, WQ-14, and WQ-15 are vegetated slopes with 0 acres of impervious area. No 152 treatment is needed.

153 Basin WQ-11 includes area which cannot drain to the South Platte River via a gravity drainage system. High flows in the South Platte River are influenced by releases from Chatfield 154 155 Reservoir. Releases from the reservoir and resulting high water surface elevations on the river 156 can occur at any time; therefore an outfall from I-25 to the river that is set too low could result in a clear day flooding of I-25 and could prevent water quality discharges to the river. A series of 157 interconnected ponds and a pump station are proposed to drain stormwater from the US 6 and 158 159 I-25 interchange. Since pumping is required, this basin is considered to be highly constrained 160 and water quality treatment is not provided. Refer to the US 6 Bridges Design Build Project 161 Master Plan Hydrology and Hydraulics Report for further discussion of this storm drain and 162 pumping system.



163 Basin WQ-2 includes a portion of the storm drain systems in US 6 between Knox Court and Federal Boulevard which discharge to Weir Gulch. Within the boundaries of this area, the 164 165 inverts of the storm drains are below the crown elevation of the Weir Gulch culvert. When the 166 culvert is running full, water will backflow into the storm system. The water quality diversions

- 167 have been placed uphill at elevations higher than the crown of Weir Gulch. Below this elevation,
- 168 stormwater drains directly to Weir Gulch and is not treated. If the diversions are placed closer to 169 the outfall to Weir Gulch in order to treat additional area, culvert flows will backflow into the
- 170 storm drain system, into the diversions, and into the US 6 Water Quality Pond. WQ-2 is not
- 171 treated.
- The storm drain system in US 6 is at a minimum 0.5% slope to the US 6 Water Quality Pond 172 from the eastbound collector/distributor ramp to Bryant Street. Basin WQ-4 represents the area 173 174 from which runoff cannot be collected, as the elevations of the ramp are lower than this storm 175 drain system. There is no room for an EDB. Maintenance of an underground BMP installed in
- 176 the traffic lanes would create a hazardous condition for the public and maintenance crews. WQ-
- 177 4 is not treated.
- 178 Basin WQ-6 represents an area at the northern extents of the project on Federal Boulevard near
- 179 7^{th} Avenue. The pavement within this basin will be overlaid, but this overlay area extends farther
- north (downstream) than the terminus of the planned curb replacement on Federal Boulevard. 180
- Inlets cannot be installed beyond the curb replacement boundaries. Therefore, runoff from this 181
- 182 area cannot be intercepted by the storm drain system and flows off-site to the north. This area is
- 183 not treated.
- 184 Basin WQ-7 is treated by the existing Barnum Lake Constructed Wetland Pond constructed with the Federal Boulevard Improvement Project: Alameda to 6th Avenue. 185
- 186 Basin WQ-8 is not treated because the elevations are lower than the storm drain system which
- diverts water quality flows to the US 6 Water Quality Pond. There is no room for an EDB. 187
- 188 Maintenance of an underground BMP installed in the traffic lanes would create a hazardous
- 189 condition for the public and maintenance crews.
- Basin WQ-9 drains to the South Platte River. A water quality diversion pipe from this system 190
- 191 cannot drain to either of the proposed EDBs at a minimum slope. There is no room for an EDB.
- Maintenance of an underground BMP installed in the traffic lanes would create a hazardous 192
- 193 condition for the public and maintenance crews. This area is not treated.
- 194 Basin WQ-10 is the ramp from eastbound US 6 to southbound I-25 and is considered highly 195 constrained. There is no room for an EDB. Maintenance of an underground BMP would create a hazardous condition for the public and maintenance crews. This area drains to the South 196 Platte River and is not treated. 197
- 198 Runoff from basin WQ-13 flows to the east project limits on US 6 and is not treated. The 199 highway is elevated in this area. Construction of an EDB or underground BMP is not possible 200 on the steep slopes. Maintenance of an underground BMP installed in the traffic lanes would 201 create a hazardous condition for the public and maintenance crews.
- 202 The untreated project impervious areas will be offset by the treated non-project areas using the 203 "offset areas" principle. The total acres of impervious area required to be treated are 73.17 204 acres. The total acres of treatment provided are 86.97 acres. The total area not treated is 23.71



207 3.4.2 US 6 Water Quality Pond

208 The US 6 Water Quality Pond will treat project and non-project impervious area from the west 209 end of the project area and will be used to provide "offset area" for portions of the project which are highly constrained and are unable to be treated. Approximately 57 acres of non-project area 210 211 (from Vrain Street to Knox Court) will flow east onto the project area. Water quality flows will be 212 diverted from WQ-1 to WQ-3 at four locations: Manholes 411R, 308R, 107C, and 111C1. The 213 water quality flows will enter the US 6 - Federal to Bryant storm drain system, will then be 214 diverted from the storm drain system at Manhole 6000C, and will outlet to the US 6 Water 215 Quality Pond on the north side of the highway. Refer to the US 6 Bridges Design Build Project 216 Master Plan Hydrology and Hydraulics Report for basin maps and storm drain system 217 schematics.

218 The US 6 Water Quality Pond will be constructed on CDOT owned property, adjacent to and on the north side of US 6 and west of Canosa Court. This pond will have a total volume of at least 219 2.45 acre-feet, which is the required WQCV for WQ-1 (1.33 acre-feet) and WQ-3 (1.12 acre-220 221 feet). The embankment should have 3:1 side slopes and the pond bottom should be graded at a 222 2% slope to the outlet. The outlet invert is at elevation 5203.00. An outlet structure with a water 223 quality plate should be designed to release water quality flows in 40 hours. The water quality 224 pond will outlet to the US 6 - Federal to Bryant Storm Drain System outfall, which outfalls to the 225 South Platte River. During large storm events, it is possible for water to backflow from the outfall 226 into the US 6 Water Quality Pond. The outlet structure will need an overflow weir in order to 227 release larger flows. An emergency spillway will be graded in on the east embankment of the 228 pond. The embankment will tie to the US 6 retaining wall on the south side of the pond, and will 229 have at least 1 foot of freeboard. The storm drain outfall into the pond is located on the south 230 side of the pond, and will require a grouted boulder rundown into a forebay in order to dissipate 231 energy. A trickle channel will convey flows at a 0.5% slope to the outlet structure.

Refer to the *Final Drainage Report for Federal Boulevard/US 6 Interchange Improvements* by
 Olsson Associates for discussion of existing drainage patterns of Federal Boulevard and 5th
 Avenue. Refer to the US 6 Bridges Design Build Project Master Plan Hydrology and Hydraulics
 Report for discussion of the storm drain system design.

- 236 See Appendix B for the Water Quality Concept Plan.
- 237 3.4.3 6th Avenue Interchange Water Quality Pond

The 6th Avenue Interchange Water Quality Pond will be constructed in CDOT right-of-way, on 238 the south side of Ramp "C," adjacent to I-25. This pond will provide WQCV for the 3rd Avenue 239 240 basin on I-25 to the south and WQ-12, for a total WQCV required of 1.01 acre-feet at elevation 5207.76. The 3rd Avenue basin will have a water quality outfall in the southeast corner of the 241 242 pond which will be constructed in the future by others. The outlet invert is at elevation 5201.23. 243 An outlet structure should be designed to release water quality flows in 40 hours and will need 244 an overflow weir for the minor and major storm event flows. The water quality pond will outlet to 245 the South Platte River. An emergency spillway will be graded in at the southwest corner of the 246 pond adjacent to Ramp "C," at elevation 5207.76.



- 247 See Appendix B for the Water Quality Concept Plan.
- 248 3.4.4 Right of Way Concerns
- 249 The proposed BMPs are located in CDOT right of way or property owned by CDOT.

250 4.0 MAINTENANCE AND OPERATION

- 4.1.1 US 6 Water Quality Pond
- The US 6 Water Quality Pond can be access from Canosa Court. The retaining wall on the north side of the highway will prevent maintenance personnel from accessing the pond from the highway.
- 255 The pond outlet works will need routine maintenance to clear the trash rack and orifice plate of
- debris. The pond bottom and trickle channel should be scraped and regraded as needed to remove sediment build-up. A minor amount of continuous maintenance will be necessary to
- keep vegetation established and not overgrown.
- 259 4.1.2 6th Avenue Interchange Water Quality Pond
- The 6th Avenue Interchange Water Quality Pond can be accessed from Ramp "C," the southbound I-25 to eastbound US 6 ramp.
- 262 The pond outlet works will need routine maintenance to clear the trash rack and orifice plate of
- 263 debris. The pond bottom and trickle channel should be scraped and regraded as needed to
- remove sediment build-up. A minor amount of continuous maintenance will be necessary to
- 265 keep vegetation established and not overgrown.



Appendix A

Water Quality Pond Stage Storage and Calculations



WQ Pond Contribution - WEST US 6 BASINS (WQ1)

Basin	Area (Ac)	%I	A*%I
WB6-7	0.42	100.00	41.9
WB6-8	0.66	100.00	65.7
EB6-5	0.74	86.57	63.7
EB6-6	0.68	84.75	57.6
OS-11	4.57	29.56	135.1
OS-12	3.28	27.22	89.2
OS-13	0.54	89.56	48.4
OS-14	1.16	24.18	28.0
OS-15	0.47	0.00	0.0
OS-16	0.78	0.00	0.0
OS-17	0.53	100.00	53.0
OS-18	0.87	0.00	0.0
OS-20	12.65	60.25	762.2
OS-21	1.87	63.10	118.0
OS-22	2.95	60.45	178.3
OS-23	2.47	52.09	128.7
OS-24	0.37	100.00	37.0
OS-25	0.61	100.00	61.0
OS-26	9.87	53.06	523.7
OS-27	5.04	100.00	504.0
OS-28	4.67	58.70	274.1
OS-29	2.11	62.77	132.4
OS-30	1.76	62.23	109.5
·	59.05	57.77	3412



Basin	Area (Ac)	%I	A*%I
SW1	0.73	100.00	73.2
SW2	0.61	56.18	34.3
SE1	0.84	97.63	82.4
SE2	1.41	100.00	140.7
P1	2.25	0.00	0.0
WB1	2.49	100.00	248.9
WB2	0.70	100.00	70.4
WB3	1.03	100.00	102.8
WB3A	0.07	100.00	6.8
WB4	1.05	100.00	104.7
WB5	0.65	100.00	65.4
WB5A	0.07	100.00	6.9
WB6-5	0.28	100.00	28.2
EB1	0.47	100.00	47.2
EB2	1.60	100.00	159.6
CD1	3.31	100.00	331.2
CD2	1.13	100.00	112.6
CD3	0.69	100.00	69.5
CD4	0.45	100.00	45.5
FR	0.69	100.00	68.8
NF1	0.23	100.00	22.7
NF2	0.29	96.51	27.6
NF3	0.36	75.62	26.9
NF4	0.40	65.62	26.3
NF5	0.25	100.00	24.7
NF6	1.11	85.73	95.5
NF7	0.54	97.95	52.7
NF8	1.02	90.56	92.1
NF9	0.32	98.76	31.9
NE	1.41	99.15	140.3
BPE1	1.83	73.10	133.8
BPE2	6.83	12.09	82.5
5TH1	1.12	69.64	78.0
5TH2	0.96	57.17	54.6
5TH3	0.47	37.07	17.6
5TH4	1.49	28.27	42.2
5TH5	1.32	28.27	37.4
5TH6	0.24	65.22	15.8
5TH7	0.10	100.00	9.9
OS-10	0.49	82.69	40.1
WEST BASINS	59.05	57.77	3411.7
	100.35	62.41	6263

<u>Water Quality Capture Volume</u> Volume Formula per UDFCD (Volume 3)

V_{WQCV}=(WQCV/12)*A*1.2

 $WQCV = 1.0^{*}(0.91^{*}I^{3}\text{-}1.19^{*}I^{2}\text{+}0.78^{*}I)$

Where: V_{WQCV} = storage volume

WQCV = watershed inches of runoff (Figure EDB-2) =
A = tributary area (acres)
1.2 Factor = Multiplier of 1.2 to account for the additional 20% of required storage for sediment accumulation

 V_{WQCV} = 2.45 ac-ft = 106,881 ft³



0.24 in

WQ Pond Contribution - WQ12 & 3rd Avenue Basin - 6th Avenue Interchange Water Quality Pond

	Basin	Area (Ac)	%I	A*%I	
_	WB6-1a	0.52	100.00	51.5	
	WB6-2a	0.15	100.00	15.3	
	EB6-1a	0.46	100.00	46.5	
	VHE4	2.66	37.96	101.1	
	VHE5	1.84	66.75	123.1	
	VHE7	0.97	100.00	97.0	
	VHW7	1.39	100.00	138.6	
	VHW6	2.86	15.66	44.8	
	RG-1	0.61	100.00	60.8	(Flyover Ramp)
	3RD	17.19	100.00	1719.0	
		28.66	83.67	2398	_

<u>Water Quality Capture Volume</u> Volume Formula per UDFCD (Volume 3)

 V_{WQCV} =(WQCV/12)*A*1.2

 $WQCV = 1.0^{\circ}(0.91^{\circ}I^{3}-1.19^{\circ}I^{2}+0.78^{\circ}I)$

Where: V_{WQCV} = storage volume

WQCV = watershed inches of runoff (Figure EDB-2) = 0.35 in A = tributary area (acres) 1.2 Factor = Multiplier of 1.2 to account for the additional 20% of required storage for sediment accumulation

$$V_{WQCV}$$
 = 1.01 ac-ft
= 44,014 ft³



STAGE-STORAGE SIZING FOR POLYGONAL, ELLIPTICAL, OR IRREGULAR PONDS



STAGE-STORAGE SIZING FOR POLYGONAL, ELLIPTICAL, OR IRREGULAR PONDS



CDOT MS4 Permit Coverage Area, Map 12



Information sources: urban area boundaries - 2000 censos from CDOT GIS Section. Jurisdictional boundaries from CDOT GIS Section Data Management Unit as of 12/31/2006, Cherry Creek Basin used to delimit MS4 area from FHU. State Hwy CDDT MS4 Coverage Area (after March 10, 2008) Local Roads

Appendix B

Water Quality Concept Plan









